FEASIBILITY REPORT UPDATE

2018 ALLEY RECONSTRUCTION PROJECT

(Originally Proposed for Construction in 2017)

City of Osseo
Hennepin County, Minnesota

NOVEMBER 9, 2017

Prepared for:
City of Osseo
415 Central Avenue
Osseo, MN 55369

WSB PROJECT NO. 3429-070

FEASIBILITY REPORT UPDATE

2018 ALLEY RECONSTRUCTION PROJECT
(Originally Proposed for Construction in 2017)

FOR THE
CITY OF OSSEO, MINNESOTA

November 9, 2017

Prepared By:

701 Xenia Avenue South | Suite 300 | Minneapolis, MN 55416 | (763) 541-4800

November 9, 2017

Honorable Mayor and City Council
City of Osseo
415 Central Avenue
Osseo, MN 55369

Re: Feasibility Report Update
2018 Alley Reconstruction Project
(Originally Proposed for Construction in 2017)
City of Osseo, MN
WSB Project No. 3429-070

Dear Honorable Mayor and City Council:

Attached for your review is an updated feasibility report which addresses improvements associated with one of the two 2018 Alley Reconstruction Projects. This project was originally presented to the City Council on December 12, 2016, and bids were received in March of 2017. Due to the high bid costs, it was recommended that Council reject the bids and rebid the project at a later date. Council concurred with these recommendations.

WSB is recommending that this project be combined with the other 2018 alley project and the 2018 street improvements in order to produce the most competitive bidding package.

We would be happy to discuss this report with you at your convenience. Please contact me at (763) 762-2821 if you have any questions or concerns.

Sincerely,

WSB & Associates, Inc.

Lee Gustafson, PE
City Engineer

Attachment

srb

CERTIFICATION

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Minnesota.

Emily A. Lueth, PE

Date: November 9, 2017 Lic. No. 51773

Quality Control Review Completed By:

Lee E. Gustafson, PE

Date: November 9, 2017 Lic. No. 18443

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1. EXECUTIVE SUMMARY

The 2018 Alley Reconstruction Project consists of repaving the alley bounded by 5th and 6th Avenues NE between 4th Street NE and 93rd Avenue, including repairing or minimal reconstruction of the storm sewer. A map illustrating the project location is shown on ***Figure 1*** in ***Appendix A***.

The deterioration of the existing alley surface and drainage issues experienced along the alley make the proposed improvements necessary. Improvements include complete reconstruction of the alley with new concrete pavement and improvements to the existing storm sewer as needed.

The total estimated project cost for this 2018 Alley Reconstruction Project is **$**246,500, and includes a 10% contingency and 25% indirect costs for legal, engineering, administrative, and financing costs. The project is proposed to be funded primarily with special assessments to benefiting property owners, as well as some City funding. The project is proposed to be completed in 2018.

The City's assessment policy calls for assessing adjacent properties 80% of the alley reconstruction costs. This cost is divided equally among the adjacent properties. The estimated special assessment to each of the 27 benefiting property owners for the alley improvements is **$**6,580**.** The City's special assessment policy calls for utility and storm sewer work to be paid from the respective enterprise fund, and as such, this proposed property assessment does not include storm sewer improvement costs.

The proposed project schedule includes construction beginning June 2018, with final completion by the fall of 2018.

This project is feasible, necessary, and cost-effective from an engineering standpoint and should be constructed as proposed herein.

2. INTRODUCTION

2.1 Authorization

On October 24, 2016, the Osseo City Council authorized a feasibility report for this 2018 Alley Reconstruction Project.

2.2 Scope

This feasibility report includes alley reconstruction and drainage improvements along the alley bounded by 5th and 6th Avenues NE between 4th Street NE and 93rd Avenue.

2.3 Data Available

Information and materials used in the preparation of this report include the following:

- City of Osseo Capital Improvement Plan
- City of Osseo Assessment Policy
- City of Osseo Property Index Records
- Storm Sewer Televising Reports
- Field Observations of the Area and Field Topography Surveys

2.4 Project History

This 2018 Alley Reconstruction Project is identified in the City's Capital Improvement Plan to address the poor condition of the alley and corresponding poor drainage.

The City held two neighborhood meetings for this project on November 10, 2016 and November 2, 2017, to receive input on the project. Many of the meeting attendees expressed support for the project, and expressed concerns about the poor drainage conditions.

3. EXISTING CONDITIONS

3.1 Surface

The existing alley consists of a deteriorated bituminous surface and spans 1,140 feet between 93rd Avenue on the north end and 4th Street NE on the south end. Many of the adjacent residents utilize the alley to access garages, parking areas, or rear yards. The project area also contains fences, driveways, and grass boulevards beyond the edge of alley, all within City right-of-way.

3.2 Drainage

Storm sewer facilities exist within the proposed project area. Generally, runoff from adjacent properties runs down the center of the alley to an existing catch basin, or to adjacent streets. Runoff that is collected in the catch basin and along the side streets ultimately flows north to the county ditch system along 93rd Avenue.

3.3 Private Utilities

Private utilities that have facilities in or near the project area will be notified during the final design phase of the project and will be requested to coordinate any necessary repairs and replacements as needed at their cost. Private utility companies that have facilities within the project area include the following:

- CenturyLink (Telephone/Internet)
- CenterPoint Energy (Gas)
- Comcast (Cable)
- Xcel Energy (Electric)
- Zayo Bandwidth (Telecom)

3.4 Watermain and Sanitary Sewer

Public utilities do not exist within the project area.

4. PROPOSED IMPROVEMENTS

4.1 Surface

The proposed surface improvements for the 2018 Alley Reconstruction Project include removal of the existing bituminous surface and installation of a new 12-foot-wide concrete pavement. Full-depth removal of the existing pavement will allow the alley to be reconstructed with a 6-inch concrete pavement on top of a 4-inch section of a Class 5 gravel base over an acceptable, compacted subgrade.

4.2 Drainage Improvements

The profile grades of the alley will be sloped to direct water to the existing catch basin. Typically, the minimum profile grade for a new street or alley is designed to a standard slope of 0.50% or greater. The alley profile will be designed to meet the minimum profile grade and maintain positive drainage at adjacent driveways and garage entrances.

4.3 Easements

It is anticipated that all alley and storm sewer work will take place within the existing alley right-of-way or within existing drainage and utility easements. Additional right-of-way acquisition is not expected to be required in order to construct the project as proposed.

4.4 Permits/Approvals

It is anticipated that no permits will be required as a part of the proposed improvements.

4.5 Public Involvement

Two neighborhood informational meetings for the proposed improvements were conducted on November 10, 2016 and November 2, 2017, for adjacent property owners. Preliminary information was presented to property owners regarding the proposed improvements including costs, funding, schedule, and project impacts. Comment cards were made available to attendees at the meeting; comments can be found in ***Appendix D***.

5. FINANCING

5.1 Opinion of Probable Cost

The total project cost is estimated at $246,500, and includes all proposed surface and storm sewer improvements as well as all engineering, legal, financing, and administrative costs. Detailed cost estimates can be found in ***Appendix B*** of this report. The opinions of cost incorporate estimated 2018 construction costs and include a 10% contingency factor. Administrative costs are projected at 25% of the construction cost and include engineering, legal, financing, and administrative costs.

5.2 Funding and Assessments

Financing the 2018 Alley Reconstruction Project will be based on the City's special assessment policy which calls for 80% of the proposed improvements to be specially assessed. The remaining 20% and 100% of the storm sewer costs will be financed by the City. Assessments for this project were calculated by dividing 80% of the total cost of the project equally among adjoining residents. There is a total of 27 properties benefiting from the improvements equating to a cost of $6,580 per unit. The proposed assessment roll is included in ***Appendix E*** of this report, along with an assessment map highlighting the benefiting properties and the assessment calculations for benefiting property owners.

6. PROJECT SCHEDULE

The proposed project schedule is as follows:

City Council Authorizes Feasibility Report.....August 28, 2017

Neighborhood Meeting.....November 2, 2017

City Council Receives Feasibility Report/Orders Public Hearing.....November 13, 2017

City Council Authorizes Final Design.....December 2017 – January 2018

Plan/Specification Preparation.....January – February 2018

City Council Approves Plans/Specs and Authorizes Bidding.....February 2018

Project Bidding.....February – March 2018

Assessment Hearing.....April 2018

City Council Awards Construction Contract.....May 2018

Construction.....June – August 2018

7. FEASIBILITY AND RECOMMENDATION

This 2018 Alley Reconstruction Project consists of reconstructing the alley bounded by 5th and 6th Avenues NE between 4th Street NE and 93rd Avenue, including repairing or minimal reconstruction of the storm sewer.

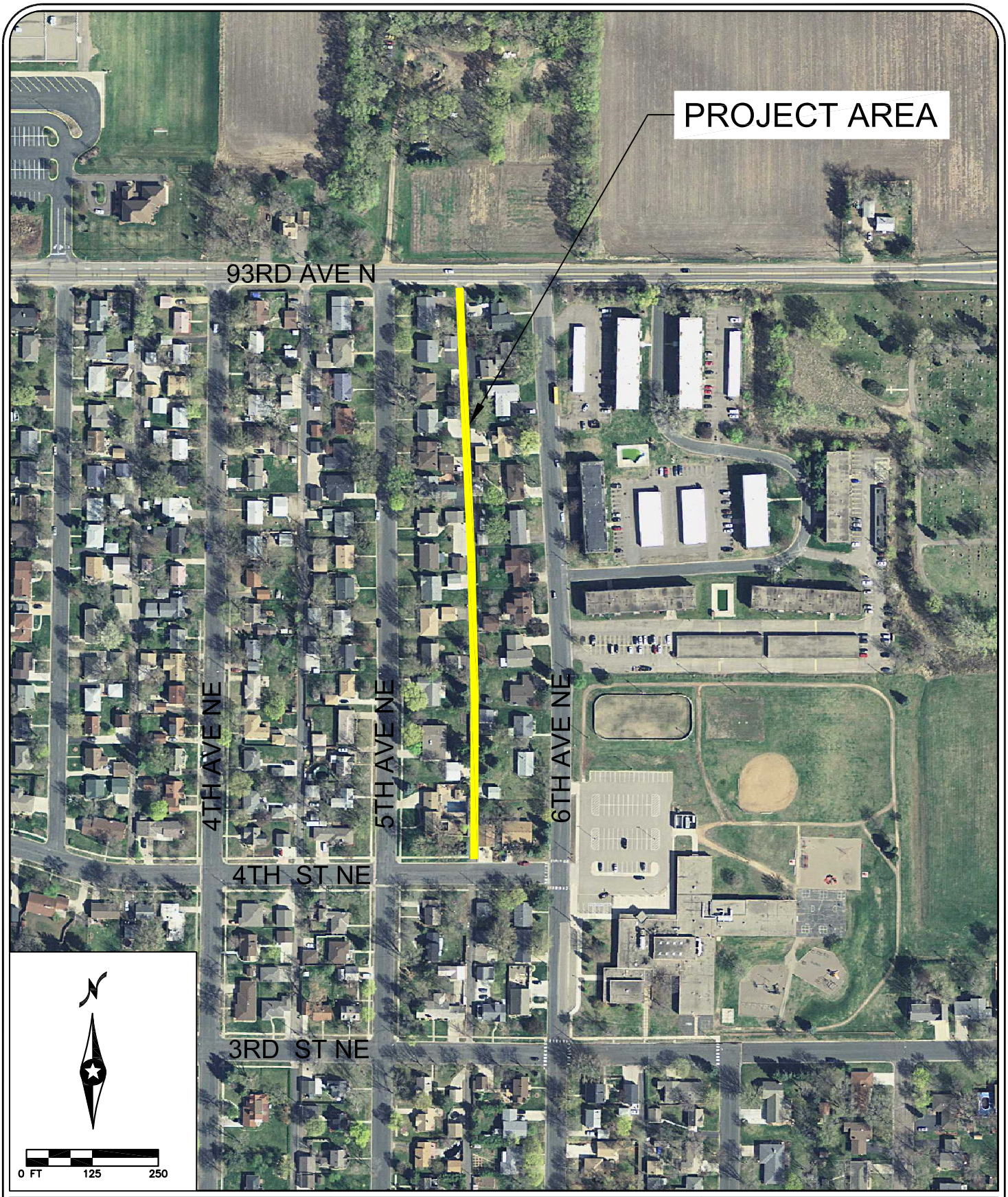
The alley totals approximately 1,140 feet (0.22 miles) and it is proposed that the aforementioned residential alley be reconstructed to a width of 12 feet.

The total estimated project cost for this 2018 Alley Reconstruction Project is $246,500. Proposed funding for the project is provided through 80% special assessments and 20% City funds. The special assessment cost to each of the 27 benefiting property owners is $6,580.

Based on the information contained within this report, the proposed improvements as described are necessary, cost-effective, and feasible from an engineering perspective. WSB & Associates, Inc. recommends construction of the proposed improvements as detailed in this report. The economic feasibility of this project will be determined by the City Council.

APPENDIX A

Figure 1 – Project Location Map
Figure 2 – Typical Section



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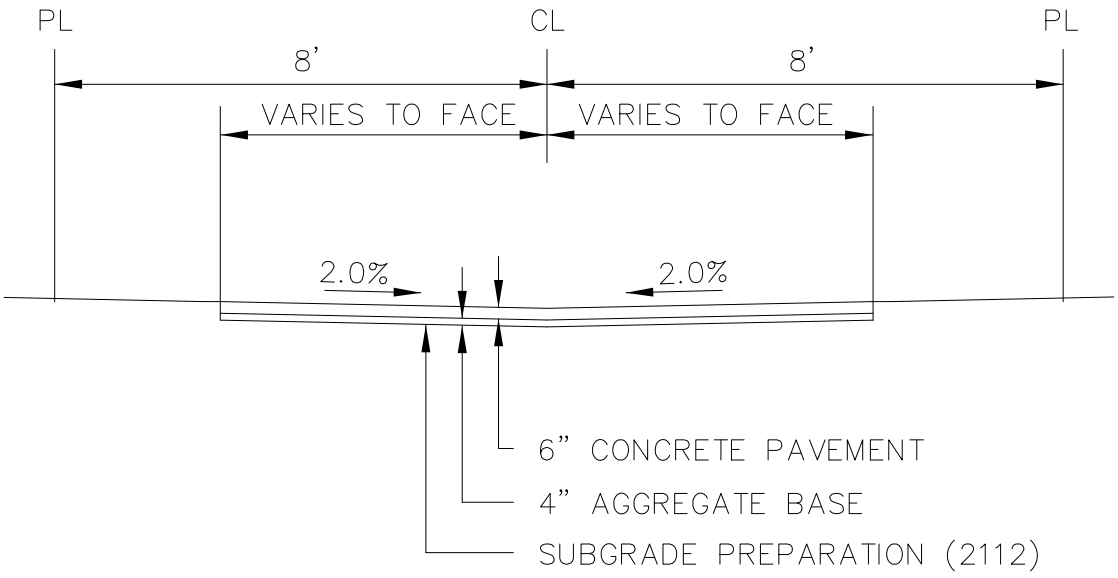
www.wsbeng.com
763-541-4800 - Fax 763-541-1700
INFRASTRUCTURE ENGINEERING PLANNING CONSTRUCTION

2018 ALLEY RECONSTRUCTION PROJECT

OSSEO, MN

11/6/2017

FIGURE 1
PROJECT AREA



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OSSEO, MN

11/6/2017

FIGURE 2
TYPICAL SECTION

APPENDIX B

Opinion of Probable Cost

Opinion of Probable Cost

WSB Project: 2018 Alley Improvement Project — *Design By:* LME
Project Location: City of Osseo — *Checked By:* EAL
WSB Project No: 03429-070 — *Date:* 11/6/2017

Item No.	MN/DOT Specification No.	Description	Unit	Estimated Total Quantity	Estimated Unit Price	Estimated Total Cost
SCHEDULE A - ALLEY IMPROVEMENTS						
1	2021.501	MOBILIZATION	LUMP SUM	1	$10,000.00	$10,000.00
2	2104.501	REMOVE CONCRETE CURB & GUTTER	LIN FT	25	$7.50	$187.50
3	2104.505	REMOVE CONCRETE DRIVEWAY PAVEMENT	SQ YD	175	$16.00	$2,800.00
4	2104.505	REMOVE BITUMINOUS DRIVEWAY PAVEMENT	SQ YD	110	$16.00	$1,760.00
5	2104.505	REMOVE BITUMINOUS PAVEMENT	SQ YD	1,465	$8.00	$11,720.00
6	2104.513	SAWING BITUMINOUS PAVEMENT (FULL DEPTH)	LIN FT	50	$8.00	$400.00
7	2104.601	SALVAGE LANDSCAPE STRUCTURES	LUMP SUM	1	$2,500.00	$2,500.00
8	2105.501	COMMON EXCAVATION (P)	CU YD	202	$30.00	$6,060.00
9	2112.501	SUBGRADE PREPARATION	RD ST	11	$250.00	$2,750.00
10	2123.610	STREET SWEEPER (WITH PICKUP BROOM)	HOUR	10	$150.00	$1,500.00
11	2130.501	WATER (DUST CONTROL)	MGAL	10	$80.00	$800.00
12	2211.501	AGGREGATE BASE CLASS 5	TON	446	$25.00	$11,150.00
13	2301.504	CONCRETE PAVEMENT 6"	SQ YD	1,486	$55.00	$81,730.00
14	2360.503	TYPE SP 12.5 WEAR COURSE MIX (2,B) 3.0" THICK	SQ YD	110	$50.00	$5,500.00
15	2505.601	UTILITY COORDINATION	LUMP SUM	1	$1,000.00	$1,000.00
16	2531.501	CONCRETE CURB AND GUTTER DESIGN B618	LIN FT	25	$60.00	$1,500.00
17	2531.507	6" CONCRETE DRIVEWAY PAVEMENT	SQ YD	175	$55.00	$9,625.00
18	2563.601	TRAFFIC CONTROL	LUMP SUM	1	$1,000.00	$1,000.00
19	2573.530	STORM INLET PROTECTION	EACH	2	$250.00	$500.00
20	2573.533	SEDIMENT CONTROL LOG TYPE STRAW	LIN FT	100	$5.00	$500.00
21	2573.533	SEDIMENT CONTROL LOG TYPE ROCK	LIN FT	50	$10.00	$500.00
22	2573.535	STABILIZED CONSTRUCTION EXIT	LUMP SUM	1	$2,000.00	$2,000.00
23	2574.525	COMMON TOPSOIL BORROW	CU YD	50	$35.00	$1,750.00
24	2575.505	SODDING TYPE LAWN	SQ YD	275	$10.00	$2,750.00
25	2575.535	WATER (TURF ESTABLISHMENT)	MGAL	30	$50.00	$1,500.00
					TOTAL	**$161,482.50**
					CONTINGENCY TOTAL (10%)	**$16,148.25**
					SUBTOTAL TOTAL	**$177,630.75**
					INDIRECT COST TOTAL (25%)	**$44,407.69**
					TOTAL	**$222,000.00**
SCHEDULE B - DRAINAGE IMPROVEMENTS						
26	2104.509	REMOVE DRAINAGE STRUCTURE	EACH	1	$1,200.00	$1,200.00
27	2503.541	12" RC PIPE SEWER DESIGN 3006 CLASS V	LIN FT	40	$80.00	$3,200.00
28	2503.602	CONNECT TO EXISTING STORM SEWER	EACH	2	$1,200.00	$2,400.00
29	2506.501	CONSTRUCT DRAINAGE STRUCTURE DES 48-4020	LIN FT	4	$1,000.00	$4,000.00
30	2506.501	CONSTRUCT DRAINAGE STRUCTURE DES 60-4020	LIN FT	5	$1,000.00	$5,000.00
31	2506.516	CASTING ASSEMBLY	EACH	2	$1,000.00	$2,000.00
					TOTAL	**$17,800.00**
					CONTINGENCY TOTAL (10%)	**$1,780.00**
					SUBTOTAL TOTAL	**$19,580.00**
					INDIRECT COST TOTAL (25%)	**$4,895.00**
					TOTAL	**$24,500.00**
					GRAND TOTAL:	**$246,500.00**

APPENDIX C

Geotechnical Report

GEOTECHNICAL REPORT

ALLEY IMPROVEMENTS

OSSEO, MN

NOVEMBER 28, 2016

Prepared for:
City of Osseo
415 Central Avenue
Osseo, Minnesota 55369

WSB PROJECT NO. 03429-060

GEOTECHNICAL REPORT

ALLEY IMPROVEMENTS
BETWEEN 5TH AND 6TH AVENUE NORTHEAST
OSSEO, MINNESOTA

FOR
CITY OF OSSEO
415 CENTRAL AVENUE
OSSEO, MINNESOTA

NOVEMBER 28, 2016

CERTIFICATION

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Minnesota.

Mark W. Osborn, PE

Date: November 28, 2016 Lic. No. 41362

540 Gateway Boulevard | Suite 100 | Burnsville, MN 55337 | (952) 737-4660

November 28, 2016

Mr. Rick Hass, Public Services Director
City of Osseo
415 Central Avenue
Osseo, Minnesota 55369

Re: Geotechnical Report
Alley Improvements
Between 5th and 6th Avenue Northeast
WSB Project No.: 03429-060

We have conducted a geotechnical subsurface exploration program for the above referenced project. This report contains our hand auger boring logs, an evaluation of the conditions encountered in the borings and our recommendations for pavement section, subgrade improvements, and other geotechnical related design and construction considerations.

If you have any questions concerning this report or our recommendations, or for construction material testing for this project, please call us at (952) 737-4660.

Sincerely,
WSB & Associates, Inc.

Mark Osborn, PE
Geotechnical Project Engineer

Darin Hyatt, PE
Senior Geotechnical Engineer

Attachment

MWO/mwo

TITLE SHEET
CERTIFICATION SHEET
LETTER OF TRANSMITTAL
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1. INTRODUCTION

1.1 Project Location

The bituminous cores and hand augers were completed within the alley that is located between 5th and 6th Avenue Northeast and connects 4th and 7th Street Northeast. The alley was a rural design bituminous asphalt surfaced roadway. The approximate cores and hand auger locations are shown on the Soil Boring Exhibit in ***Appendix A***.

1.2 Project Description

It is proposed to reconstruct the alley and place a new bituminous asphalt pavement surface. The alley will remain a rural design section. We understand that no underground utilities will be constructed within the alley. We understand the horizontal and vertical alignments of the roadway will not be altered from existing conditions.

WSB has developed recommendations for this project in consideration of the proposed layout and configurations as understood at this time. WSB must be made aware of the revised or additional information in order to evaluate the recommendations for continued applicability.

1.3 Purpose and Project Scope of Services

The City of Osseo (City) authorized this work. In order to assist the design team in preparing plans and specifications, we have developed recommendations for pavements and subgrades. As such, we have completed a subsurface exploration program and prepared a geotechnical report for the referenced site. This stated purpose was a significant factor in determining the scope and level of service provided. Should the purpose of the report change the report immediately ceases to be valid and use of it without WSB's prior review and written authorization shall be at the user's sole risk.

Our authorized scope of work has been limited to:

1. Mobilization / Demobilization of a Coring Crew.
2. Clearing underground utilities utilizing the Gopher State One Call.
3. Coring three (3) locations and completing hand augers to 5 foot depths.
4. Perform soil classification and analysis.
5. Review of readily available project information and geologic data.
6. Providing this geotechnical report containing:
    - A. Summary of our findings.
    - B. Discussion of subsurface soil and groundwater conditions and how they may affect the proposed pavements.
    - C. Estimated R-value of the soils.
    - D. Recommended pavement section.
    - E. A discussion of soils for use as site fills.

2. PROCEDURES

2.1 Boring Layout and Soil Sampling Procedures

The City requested we complete three (3) soil borings along the alleyway. After our site visit to layout boring locations and meet with utility locators, we noted that the overhead power lines and narrow alleyway would prevent our drill rig from being able to perform soils borings. We contacted the Client and it was agreed that three (3) bituminous cores and hand augers would be completed along the alley. WSB recommended the depths and selected the desired locations. Our field crew staked the borings from existing site features from the supplied site plan. The approximate boring locations are shown on the Soil Boring Exhibit in ***Appendix A*** which is an aerial photo.

We completed the cores and hand augers on November 8, 2016 with a coring machine with a 6 inch barrel and a 1¼" steel hand auger. Methods, crew chief, depths, sampling interval, groundwater observations, test data, and other information are indicated on the hand auger boring logs.

The materials encountered were described on field logs and representative samples were containerized, and transported to our laboratory for further examination and testing.

The samples were visually examined to estimate the distribution of grain sizes, plasticity, consistency, moisture condition, color, presence of lenses and seams, and apparent geologic origin. We classified the soils according to type using the Unified Soil Classification System (USCS). A chart describing the Unified Soil Classification System is included in ***Appendix A***.

2.2 Groundwater Measurements and Borehole Abandonment

The crew observed the borings for free groundwater after completion. These observations and measurements are noted on the boring logs. The crew then backfilled with soil cuttings.

2.3 Boring Log Procedures and Qualifications

The subsurface conditions encountered by the test borings are illustrated on the Logs of Test Borings in ***Appendix A***. Similar soils were grouped into the strata shown on the boring logs, and the appropriate estimated USCS classification symbols were also added. The depths and thickness of the subsurface strata indicated on the boring logs were estimated from the auger results.

The transition between materials (horizontal and vertical) is approximate and is usually far more gradual than shown. Information on actual subsurface conditions exists only at the specific locations indicated and is relevant only to the time exploration was performed. Subsurface conditions and groundwater levels at other locations may differ from conditions found at the indicated locations. The nature and extent of these conditions would not become evident until exposed by construction excavation. These stratification lines were used for our analytical purposes and, due to the aforementioned limitations, should not be used as a basis of design or construction cost estimates.

3. EXPLORATION RESULTS

3.1 Site and Geology

The cores and augers were completed through the existing bituminous asphalt pavement section and encountered mostly fills overlying lean clays and clayey sands.

The Hennepin County Geologic Atlas indicates the surficial geology of the area is mostly sand and gravel.

3.2 Subsurface Soil and Groundwater Conditions

The boring profile generally consisted of pavement section materials and fills overlying fine and mixed alluvial soils.

Fill materials were encountered to depths of 1 foot at Boring HA-3 and extended to termination at Borings HA-1 and HA-2. These fills consisted mostly of silty sands and lean clays.

Lean clays were encountered below the fills at Boring HA-3, and were generally brown in color and moist. Clayey sands were encountered below the clays and were generally brown in color and moist.

Table 1 below indicates the soils present within the upper 4 feet of the roadway profile. These soils typically have the greatest effect on the roadway subgrade.

Table 1: Roadway Soil Boring Profiles

Boring No.	Bituminous Thickness (inches)	Aggregate Base Thickness (inches)	Subgrade Soils (Upper 4 feet)
PB-1	2	10	Fill (Silty Sand and Lean Clay)
PB-2	1½	10½	Fill (Silty Sand)
PB-3	2	10	Lean Clay, Clayey Sand

3.3 Groundwater Conditions

WSB took groundwater level readings in the exploratory auger holes, reviewed the data obtained, and discussed its interpretation of the data in the text of the report. Note that groundwater levels may fluctuate due to seasonal variations (e.g. precipitation, snowmelt and rainfall) and/or other factors not evident at the time of measurement.

No groundwater was encountered during the augers. The holes were only left open for a short time during the process.

4. ENGINEERING ANALYSIS AND RECOMMENDATIONS

4.1 Discussion

The fills encountered onsite consisted of lean clays and silty sands. These fills have been inplace for a long period of time and were likely placed to raise the roadway to grade or as utility backfill. Primary settlement of these soils should be complete. Based on this, it is our opinion these soils are suitable to remain in place as subgrade soils.

The native lean clays and clayey sands would be suitable for pavement support.

The soils within the pavement subgrade consist of clays and silty sands, which are frost susceptible soils. Consideration should be given to partially subcutting these soils and replacing with a non-frost susceptible granular fill to reduce the potential frost heave below the pavement section.

4.2 Pavement Areas

After excavation of the pavement section, proof-roll tests should be utilized with a loaded dump truck to help identify areas that may require corrective action such as scarifying, disking, and compaction or sub-excavations. We also recommend a proof-roll be performed again on the aggregate base just prior to placement of the bituminous pavement.

Once the site has been prepared as recommended, we anticipate the subgrade will consist of a mixture of lean clays, clayey sands, and silty sands. The MnDOT Flexible Pavement Design Guidance Memo from June 2014, indicates soils such as those anticipated have R-values ranging from about ten to thirty (10-30). We recommend an R-value of fifteen (15) be used for design of roadways at this site.

No traffic data was available for the alleyway. We would expect traffic to be limited to residents driving to their garage and for garbage trucks. We estimated the Average Daily Traffic (ADT) to be less than 200 vehicles. Our design is based on a twenty (20) year design life of the pavement section.

Based on the design R-value and the traffic ADT we calculated the roadway traffic to be equivalent to approximately 18,000 Estimated Single Axle Loads (ESAL's).

Bituminous Pavement Option

Based on MnDOT's MnPave software, we recommend the pavement section in Table 2.

Table 2: Recommended Flexible Pavement Section

Section	Thickness (inches)
Bituminous Wear Course, MnDOT 2360	1½
Bituminous Base Course, MnDOT 2360	2
MnDOT Class 5 Aggregate Base	6

As previously mentioned the use of a non-frost susceptible sand cushion will help reduce the effects of frost heave. In our opinion, a 30 inch sand cushion typically provides adequate reduction in frost heave potential. It should be noted that any sand cushion placed below the pavement section will provide positive benefits for reduced potential frost heave.

Drainage of the sand cushion via drain tile may be necessary. Drain tile wrapped in a sock should be placed at the base of the sand cushion and tied into catch basins. We recommend the sand cushion contain less than ten percent (10%) passing the #200 sieve.

Within several years after initial paving, some thermal shrinkage cracks will develop. We recommend routine maintenance be performed to improve pavement performance and increase pavement life. Pavement should be sealed with a liquid bitumen sealer to retard water intrusion into the base course and subgrade. Localized patch failures may also develop where trucks or buses turn on the pavement. When these occur, they should be cut out and patch repaired. Periodic seal coating should also be applied, to preserve the longevity of the pavement.

Concrete Pavement Option

Alternately, a concrete section could be designed for these roadways. The concrete section in Table 3 is based on minimum recommendations from MnDOT design methods.

Table 3: Commercial Roadways
Concrete – Rigid Pavement Section

Section	Thickness (inches)
Concrete, MnDOT	6
MnDOT Class 5 Aggregate Base	4

MnDOT recommends joint spacing of twelve foot (12') or fifteen foot (15') lengths. We used twelve feet (12') for our calculations in the MnDOT Rigid Pavement Design software. We suggest the concrete mix be air-entrained to six percent plus or minus one and one-half percent (6% +/- 1½ %) with a minimum twenty-eight (28) day compressive strength of 4,000 psi. The slump range should be between two to five inches (2-5") to reduce shrinkage related problems such as curling or excessive cracking. A maximum water to cement ratio of forty-five hundredths (0.45) is recommended. Type 1 cement should be acceptable, and should meet the requirements of ASTM C150. The concrete mix design should follow MnDOT requirements.

General Design

The pavement sections above provide options to meet the ESAL requirements. Other pavement design options would be acceptable as well as long as they meet the minimum requirements for bituminous/concrete thickness, aggregate base thickness, and can meet the ESAL requirements.

4.3 Backfill and Fill Selection and Compaction

The on-site non-organic soils may be reused as backfill and fill provided they are moisture conditioned and can be compacted to their specified densities. Any wet soils excavated would need to be dried before reuse as an engineered fill. Backfills with cobbles larger than six inches (6") should not be placed below pavements or in contact with utilities. We recommend that sandy soils be moisture conditioned to meet compaction specifications and clayey soils be moisture conditioned to within two percent (2%) below to three percent (3%) above their optimum moisture contents as determined from their standard Proctor tests (ASTM D-698). Fill should be spread in thin lifts to allow for complete compaction of the material. Table 4 indicates the recommended compaction levels.

Table 4: Recommended Level of Compaction for Backfill and Fill

Area	Percent of Standard Proctor Maximum Dry Density
Pavement: Within 3 feet of top of subgrade	100
Pavement: Greater than 3 feet below top of subgrade	95
Utility Trench	95
Landscaping (non-structural)	90

4.4 Construction Considerations

Good surface drainage should be maintained throughout the work so that the site is not vulnerable to ponding during or after a rainfall. The excavation for any soil correction to densify loose fill should not encounter groundwater intrusion. However, if water does enter excavations, it should be promptly removed prior to further construction activities. Under no circumstances should fill or concrete be placed into standing water. Trenches for underground utility lines serving the building addition are also expected to be dry.

Soil corrections at this site for pavement subgrades may not be continuous in all areas. We recommend tapering the fills back to native soils at a ten to one (10:1) slope.

4.5 Construction Safety

All excavations must comply with the requirements of OSHA 29 CFR, Part 1926, Subpart P "Excavations and Trenches". This document states that excavation safety is the responsibility of the contractor. Reference to this OSHA requirement should be included in the job specifications.

The responsibility to provide safe working conditions on this site, for earthwork, building construction, or any associated operations is solely that of the contractor. This responsibility is not borne in any manner by WSB.

4.6 Cold Weather Construction

It is our understanding that construction is unlikely to occur during the winter months. However, if the construction does continue into the winter months we recommend the following guidelines.

Only unfrozen fill should be used. Placement of fill or concrete *must not be permitted* on frozen soil.

4.7 Field Observation and Testing

The soil conditions illustrated on the Logs of Test Borings in ***Appendix A*** are indicative of the conditions only at the boring locations.

WSB also recommends a representative number of field density tests be taken in all engineered fill and backfill placed to aid in judging its suitability. Fill placement and compaction should be monitored and tested to determine that the resulting fill and backfill conforms to specified density, strength or compressibility requirements. Prior to use, any proposed fill and backfill material should be submitted to the WSB laboratory for testing to verify compliance with recommendations and project specifications.

Dynamic Cone Penetrometer (DCP) tests can be completed in the aggregate base in lieu of density testing. We recommend following MnDOT Specification 2211-3.

WSB would be pleased to provide the necessary field observation, monitoring and testing services during construction.

4.8 Plan Review and Remarks

The observations, recommendations and conclusions described in this report are based primarily on information provided to WSB, obtained from our subsurface exploration, our experience, several necessary assumptions and the scopes of service developed for this project and are for the sole use of our client. We recommend that WSB be retained to perform a review of final design drawing and specifications to evaluate that the geotechnical engineering report has not been misinterpreted. Should there be any changes in the design related to this project or if there are any uncertainties in the report we should be notified. We would be pleased to review any project changes and modify the recommendations in this report (if necessary) or provide any clarification in writing.

The entire report should be kept together; for example, boring logs should not be removed and placed in the specifications separately.

The boring logs and related information included in this report are indicators of the subsurface conditions only at the specific locations indicated on the Soil Boring Exhibit and times noted on the Logs of Test Boring sheets in ***Appendix A***. The subsurface conditions, including groundwater levels, at other locations on the site may differ significantly from conditions that existed at the time of sampling and at the boring locations.

The test borings were put down by WSB solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions.

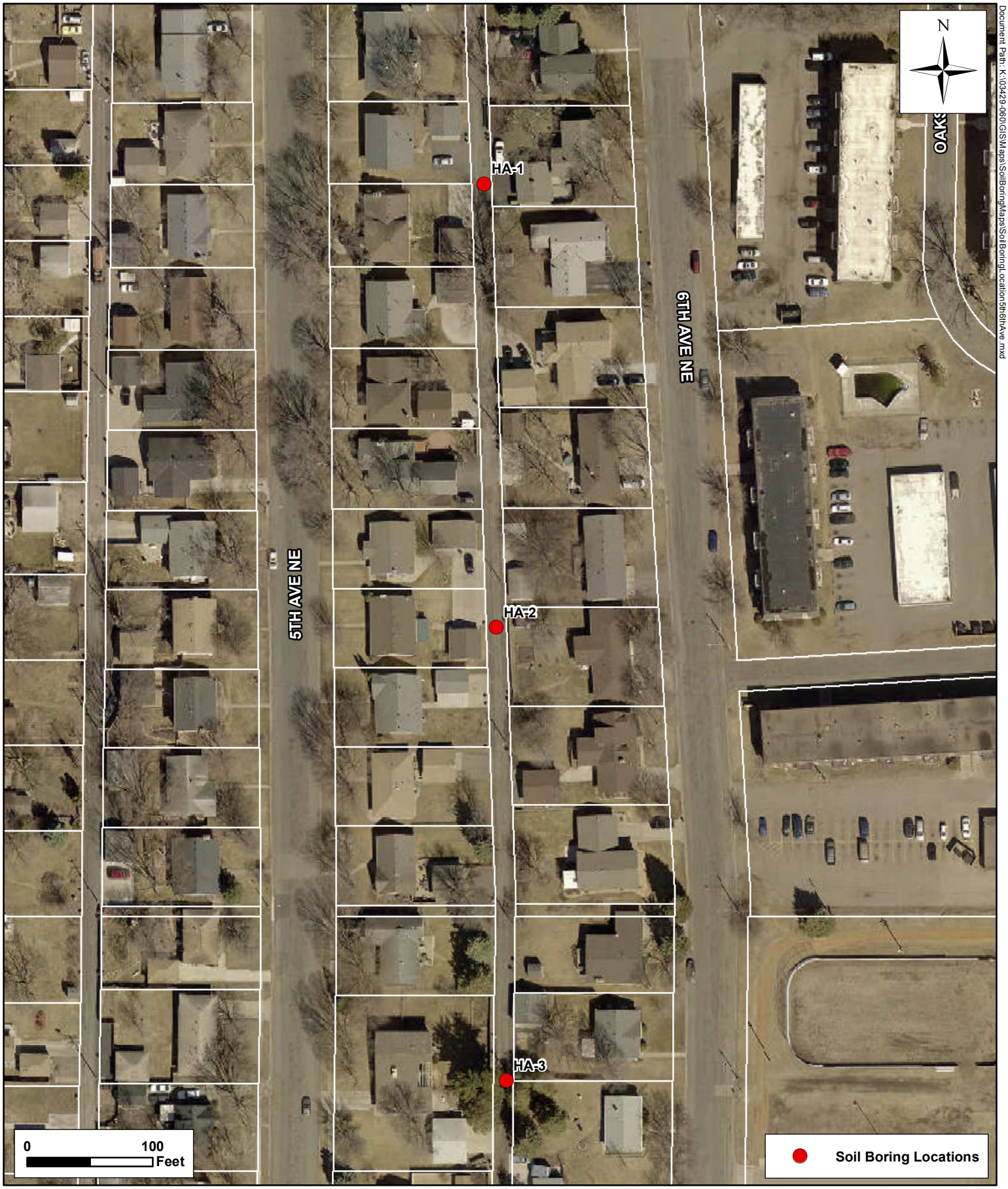
WSB has not performed any observations, investigations, studies or testing that is not specifically listed in the scope of service. WSB shall not be liable for failing to discover any condition whose discovery required the performance of services not authorized by the Agreement.

5. STANDARD OF CARE

The recommendations and opinions contained in this report are based on our professional judgment. The soil testing and geotechnical engineering services performed for this project have been performed with the level of skill and diligence ordinarily exercised by reputable members of the same profession under similar circumstances, at the same time and in the same or a similar locale. No warranty, either express or implied, is made.

APPENDIX A

Soil Borings Exhibit
Logs of Test Borings
Symbols and Terminology on Test Boring Log
Notice to Report Users Boring Log Information
Unified Soil Classification Sheet (USCS)



Document Path: K:\03429-060\GIS\Maps\SoilBoringMaps\SoilBoringLocation5th6thAve.mxd

Soil Boring Exhibit
Alley Between 5th and 6th Avenue NE
Osseo, Minnesota

Osseo EST. 1875

WSB

WSB# 03429-060

LOG OF TEST BORING

PROJECT NAME: 2017 Street Improvements
PROJECT LOCATION: Osseo, MN
CLIENT/WSB #: 03429-060

BORING NUMBER HA 1

DEPTH (ft)	DESCRIPTION OF MATERIAL	USCS	GEOLOGIC ORIGIN		WL	SAMPLE No.	SAMPLE TYPE	LABORATORY TESTS MC (%)	DD (pcf)	LL (%)	PL (%)	
	BITUMINOUS 0 - 2" AGGREGATE BASE 2" - 12"		Pavement Section			1	AU					
1	FILL, mostly Silty Sand, dark brown		Fill									
2						2	AU					
3	FILL, mostly Lean Clay, grayish brown		Fill									
4						3	AU					
5	End of Boring 5.0 ft.											
6												
7												
8												
9												
10												
11												

WATER LEVEL MEASUREMENTS

START: 11/08/2016 END: 11/08/2016

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WATER ELEVATION	METHOD	Crew Chief:	Logged By:
								J. Tatro	DAJ
11/08/2016		5			None		1 1/2" HA 0' - 5'	Notes:	

LOG OF TEST BORING

PROJECT NAME: 2017 Street Improvements
PROJECT LOCATION: Osseo, MN
CLIENT/WSB #: 03429-060

BORING NUMBER HA 2

DEPTH (ft)	DESCRIPTION OF MATERIAL	USCS	GEOLOGIC ORIGIN		WL	SAMPLE No.	SAMPLE TYPE	LABORATORY TESTS MC (%)	DD (pcf)	LL (%)	PL (%)	
	BITUMINOUS 0 - 1 1/2" AGGREGATE BASE 1 1/2" - 12"		Pavement Section			1	AU					
1	FILL, mostly Silty Sand, dark brown		Fill									
2												
3						2	AU					
4												
5	End of Boring 5.0 ft.											
6												
7												
8												
9												
10												
11												

WATER LEVEL MEASUREMENTS

START: 11/08/2016 END: 11/08/2016

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WATER ELEVATION	METHOD
11/08/2016		5			None		1 1/2" HA 0' - 5'

Crew Chief: J. Tatro
Logged By: DAJ
Notes:

LOG OF TEST BORING

PROJECT NAME: 2017 Street Improvements
PROJECT LOCATION: Osseo, MN
CLIENT/WSB #: 03429-060

BORING NUMBER HA 3
PAGE 1 OF 1

DEPTH (ft)	DESCRIPTION OF MATERIAL	USCS	GEOLOGIC ORIGIN	WL	SAMPLE No.	SAMPLE TYPE	LABORATORY TESTS MC (%)	DD (pcf)	LL (%)	PL (%)
	BITUMINOUS 0 - 2" AGGREGATE BASE 2" - 12"		Pavement Section		1	AU				
1	LEAN CLAY, brown, moist	CL	Fine Alluvium							
2					2	AU				
3	CLAYEY SAND, brown, moist	SC	Mixed Alluvium							
4					3	AU				
5	End of Boring 5.0 ft.									
6										
7										
8										
9										
10										
11										

WATER LEVEL MEASUREMENTS

START: 11/08/2016
END: 11/08/2016

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER DEPTH	WATER ELEVATION	METHOD
11/08/2016		5			None		1 1/2" HA 0' - 5'

Crew Chief: J. Tatro
Logged By: DAJ
Notes:

WSB BORING LOG - WSB.GDT - 11/21/16 09:21 - K:\03429-060\GEOTECH-CMT\2017 STREET IMPROVEMENTS.GPJ

SYMBOLS AND TERMINOLOGOY ON TEST BORING LOG

SYMBOLS

Drilling and Sampling

Symbol	Description
HSA	3-1/4" LD. Hollow stem auger
_ FA	4", 6" or 10" diameter flight auger
_HA	2", 4", or 6" hand auger
_DC	2-1/2", 4", 5", or 6" steel drive casing
_RC	Size A, B or N rotary casing
PD	Pipe drill or cleanout tube
CS	Continuous split barrel sampling
DM	Drilling mud
JW	Jetting water
SB	2" O.D. split barrel sampling
_L	2-1/2" or 3-1/2" O.D. SB liner sampler
_T	2" or 3" thin walled tube sample
3TP	3" thin walled tube using pitcher sampler
_TO	2" or 3" thin walled tube using Osterberg sampler
W	Wash sample
B	Bag sample
P	Test pit sample
_Q	BQ, NQ, or PQ wire line system
_X	AX, BX, or NX double tube barrel
N	Standard penetration test, blows per foot
CR	Core recovery, percent
WL	Water level
▼	Water level
NMR	No measurement recorded, primarily due to presence of drilling or coring fluid.

Laboratory Testing

Symbol	Description
W	Water content, % (ASTM** D2216)
D	Dry density, pcf
LL	Liquid limit (ASTM D4318)
PL	Plastic limit (ASTM D4318)
	-Inserts in last column (Qu or RQD)-
Qu	Unconfined compressive strength, psf (ASTM D2166)
Pq	Penetrometer reading, tsf (ASTM D1558)
Ts	Torvane reading, tsf
G	Specific gravity (ASTM D854)
SL	Shrinkage limits (ASTM D427)
OC	Organic content-combustion method (ASTM D2974)
SP	Swell pressure, tsf (ASTM D4546)
PS	Percent swell under pressure (ASTM D4546)
FS	Free swell, % (ASTM D4546)
SS	Shrink swell, % (ASTM D4546)
pH	Hydrogen ion content-Meter Method (ASTM D4972)
SC	Sulfate content, parts/million or mg/l
CC	Chloride content, parts/million or mg/l
C*	One dimensional consolidation (ASTM D2435)
Qc*	Triaxial compression (ASSTM D2850 and D4767)
D.S.*	Direct Shear (ASTM D3080)
K*	Coefficient of permeability, cm/sec (ASTM D2434)
P*	Pinhole test (ASTM D4647)
DH*	Double hydrometer (ASTM D4221)
MA*	Particle size analysis (ASTM D422)
R	Laboratory electrical resistivity, ohm-cm (ASTM G57)
E*	Pressuremeter deformation modulus, tsf (ASTM D4719)
PM*	Pressuremeter test (ASTM D4719)
VS*	Field vane shear (ASTM D2573)
IR*	Infiltrometer test (ASTM D3385)
RQD	Rock quality designation, percent

*Results shown on attached data sheet or graph

**ASTM designates American Society for Testing and Materials

TERMINOLOGY

Particle Sizes

Type	Size Range
Boulders	> 12"
Cobbles	3" – 12"
Coarse gravel	3/4" – 3"
Fine gravel	#4 sieve – 3/4"
Coarse sand	#4 - #10 sieve
Medium sand	#10-#40 sieve
Fine sand	#40-#200 sieve
Silt	100% passing #200 sieve and > 0.005mm
Clay	100% passing #200 sieve and < 0.005mm

Soil layering and Moisture

Term	Visual Observation
Lamination	Up to 1/4" thick stratum
Varved	Altering laminations of any combination of clay, silt, fine sand, or colors
Lenses	Small pockets of different soils in a soil mass
Stratified	Altering layers of varying materials or colors
Layer	1/4" to 12" thick stratum
Dry	Powdery, no noticeable water
Moist	Damp, below saturation
Waterbearing	Pervious soil below water
Wet	Saturated, above liquid limit

Gravel Content

Coarse-Grained Soils

% Gravel	Description
2-15	A little gravel
16-49	With gravel

Fine-Grained Soils

% Gravel	Description
< 5	Trace of gravel
5-15	A little gravel
16-30	With gravel
31-49	Gravelly

Standard Penetration Resistance

Cohesionless Soils

N-Value	Relative Density
0-4	Very loose
5-10	Loose
11-30	Medium dense
31-50	Dense
> 50	Very dense

Cohesive Soils

N-Value	Consistency
0-4	Very soft
5-8	Soft
9-15	Firm
16-30	Hard
> 30	Very hard

NOTICE TO REPORT USERS BORING LOG INFORMATION

Subsurface Profiles

The subsurface stratification lines on the graphic representation of the test borings show an approximate boundary between soil types or rock. The transition between materials is approximate and is usually far more gradual than shown. Estimating excavation depths, soil volumes and other computations relying on the subsurface strata may not be possible to any degree of accuracy.

Water Level

WSB & Associates, Inc. took groundwater level readings in the exploratory borings, reviewed the data obtained, and discussed its interpretation of the data in the text of this report. The groundwater level may fluctuate due to seasonal variations caused by precipitation, snowmelt, rainfalls, construction or remediation activities, and/or other factors not evident at the time of measurement.

The actual determination of the subsurface water level is an interpretative process. Subsurface water level may not be accurately depicted by the levels indicated on the boring logs. Normally, a subsurface exploration obtains general information regarding subsurface features for design purposes. An accurate determination of subsurface water levels is not possible with a typical scope of work. The use of the subsurface water level information provided for estimating purposes or other site review can present a moderate to high risk of error.

The following information is obtained in the field and noted under "Water Level Measurements" at the bottom of the log.

Sampled Depth: The lowest depth of soil sampling at the time a water level measurement is taken.

Casing Depth: The depth to the bottom of the casing or hollow-stem auger at the time of water level measurement.

Cave-In Depth: The depth at which the measuring tape stops in the bore hole.

Water Level: The point in the bore hole at which free-standing water is encountered by a measuring tape dropped from the surface inside the casing.

Drilling Fluid Level: Similar to the water level, except the liquid in the bore hole is a drilling fluid.

Obstruction Depths

Obstructions and/or obstruction depths may be noted on the boring logs. Obstruction indicates the sampling equipment encountered resistance to penetration. It must be realized that continuation of drilling, the use of other drilling equipment or further exploration may provide information other than that depicted on the logs. The correlation of obstruction depths on the log with construction features such as rock excavation, foundation depths, or buried debris cannot normally be determined with any degree of accuracy. For example, penetration of weathered rock by soil sampling equipment may not correlate with removal by certain types of construction equipment. Using this information for estimating purposes often results in a high degree of misinterpretation.

Accurately identifying the obstruction or estimating depths where hard rock is present over the site requires a scope of service beyond the normal geotechnical exploration program. The risk of using the information noted on the boring logs for estimating purposes must be understood.

WSB & Associates, Inc.

UNIFIED SOIL CLASSIFICATION SYSTEM

UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART

COARSE-GRAINED SOILS (more than 50% of material is larger than No. 200 sieve size.)			
GRAVELS More than 50% of coarse fraction larger than No. 4 sieve size	Clean Gravels (Less than 5% fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines
		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
	Gravels with fines (More than 12% fines)	GM	Silty gravels, gravel-sand-silt mixtures
		GC	Clayey gravels, gravel-sand-clay mixtures
SANDS 50% or more of coarse fraction smaller than No. 4 sieve size	Clean Sands (Less than 5% fines)	SW	Well-graded sands, gravelly sands, little or no fines
		SP	Poorly graded sands, gravelly sands, little or no fines
	Sands with fines (More than 12% fines)	SM	Silty sands, sand-silt mixtures
		SC	Clayey sands, sand-clay mixtures
FINE-GRAINED SOILS (50% or more of material is smaller than No. 200 sieve size.)			
SILTS AND CLAYS Liquid limit less than 50%		ML	Inorganic silts and very fine sands, rock flour, silty of clayey fine sands or clayey silts with slight plasticity
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		OL	Organic silts and organic silty clays of low plasticity
SILTS AND CLAYS Liquid limit 50% or greater		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
		CH	Inorganic clays of high plasticity, fat clays
		OH	Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS		PT	Peat and other highly organic soils

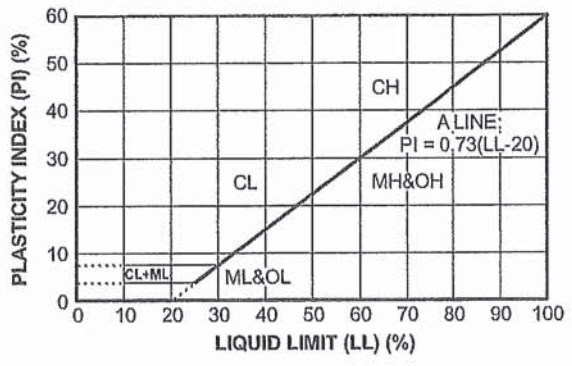
LABORATORY CLASSIFICATION CRITERIA

GW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
GP	Not meeting all gradation requirements for GW	
GM	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
GC	Atterberg limits above "A" line with P.I. greater than 7	
SW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
SP	Not meeting all gradation requirements for GW	
SM	Atterberg limits below "A" line or P.I. less than 4	Limits plotting in shaded zone with P.I. between 4 and 7 are borderline cases requiring use of dual symbols.
SC	Atterberg limits above "A" line with P.I. greater than 7	

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:

Less than 5 percent ..... GW, GP, SW, SP
More than 12 percent ..... GM, GC, SM, SC
5 to 12 percent ..... Borderline cases requiring dual symbols

PLASTICITY CHART



APPENDIX D

Televising Report

Televising Report

for

WSB & Associates

Televising

in

Osseo, MN

November 23rd, 2016

HK HYDRO KLEAN

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Hydro-Klean, LLC
21040 Commerce Blvd.
Rogers, Minnesota 55374
763.428.6488 | fax: 763.428.6489
www.hydro-klean.com

Tabular Report of PSR ST 8 for OSSEO

Setup 20	**Surveyor** Kyle Van Pelt	**Certificate #** V3516	**System Owner**		
Drainage	**Survey Customer**				
P/O #	**Date** 2016/11/11	**Time** 10:38	**Street** 5th Ave NE		
City Osseo	**Further location details**				
Up ST 8	**Rim to invert**	**Grade to invert**	**Rim to grade**	**Ft**	
Down ST 9	**Rim to invert**	**Grade to invert**	**Rim to grade**	**Ft**	
Use Stormwater	**Direction** Down	**Flow control**	**Media No**		
Shape Circular	**Height** 15 **Width**	**ins** **Preclean** J	**Date Cleaned**		
Material Vitrified Clay Pipe	**Joint length** **Ft**	**Total length** 133.8 Ft	**Length Surveyed** 133.8 **Ft**		
Lining	**Year laid**	**Year rehabilitated**	**Weather**		
Purpose	**Cat**		**Pressure** 0.000		
Additional info			Structural O & M Constructional Miscellaneous Hydraulic		
Location					
Project			**Work Order**		
Northing	**Easting**		**Elevation**		
Coordinate System			**GPS Accuracy**		

Count	Video	CD	Code		In1	In2	%	Jnt	Fr	To	ImRef	Remarks
0.0			ST	Start of Survey								
0.0			AMH	Manhole								
0.0			MWL	Water Level			5					
133.8			AMH	Manhole								
133.8			FH	End of Survey								

133.8 **Ft** **Total Length Surveyed**

Scores

Structural:	**Pipe Rating** 0	**Pipe Ratings Index** 0	**Peak** 0	**Mean Pipe** 0
O&M:	**Pipe Rating** 0	**Pipe Ratings Index** 0	**Peak** 0	**Mean Pipe** 0

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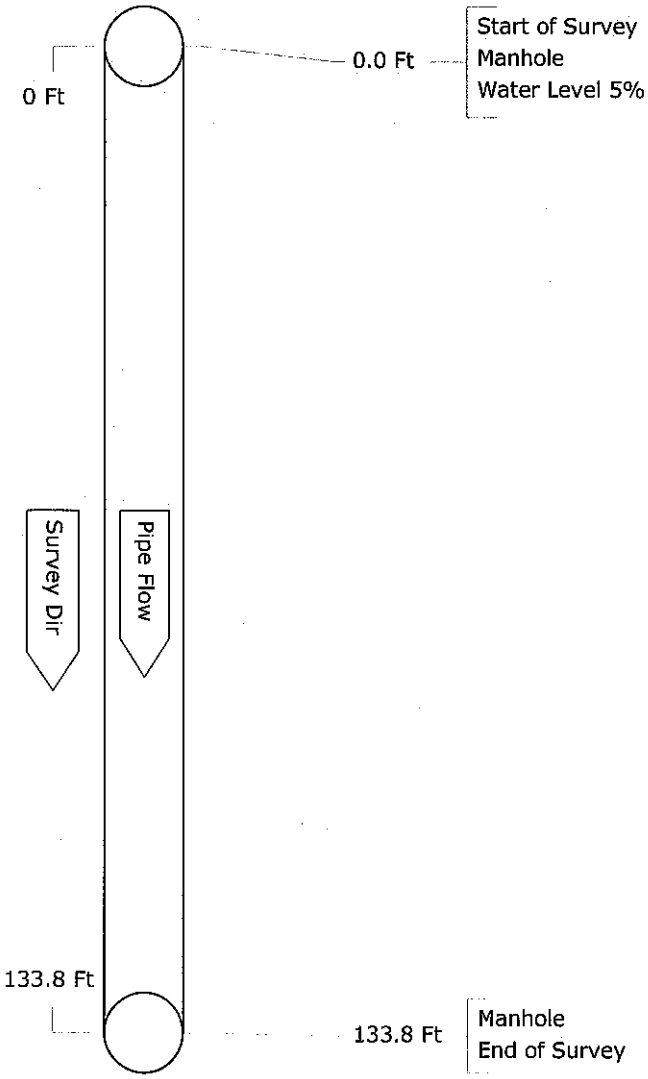
Phone:763-428-6488
Fax:763-428-6489

Pipe Graphic Report of PSR ST 8 for OSSEO

Setup 20	**Surveyor** Kyle Van Pelt	**Certificate #** V3516	**System Owner**		
Drainage	**Survey Customer**				
P/O #	**Date** 2016/11/11	**Time** 10:38	**Street** 5th Ave NE		
City Osseo	**Further location details**				
Up ST 8	**Rim to invert**	**Grade to invert**	**Rim to grade**	**Ft**	
Down ST 9	**Rim to invert**	**Grade to invert**	**Rim to grade**	**Ft**	
Use Stormwater	**Direction** Downstream	**Flow control**	**Media No**		
Shape Circular	**Height** 15 **Width**	**ins** **Preclean** J	**Date Cleaned**		
Material Vitrified Clay Pipe	**Joint length** **Ft**	**Total length** 133.8 **Ft**	**Length Surveyed** 133.80 **Ft**		
Lining	**Year laid**	**Year rehabilitated**	**Weather**		
Purpose	**Cat**				
Additional info		Structural	O & M	Constructional	
Location		Miscellaneous	Hydraulic		
Project		**Work Order**			
Northing	**Easting**	**Elevation**			
Coordinate System		**GPS Accuracy**			



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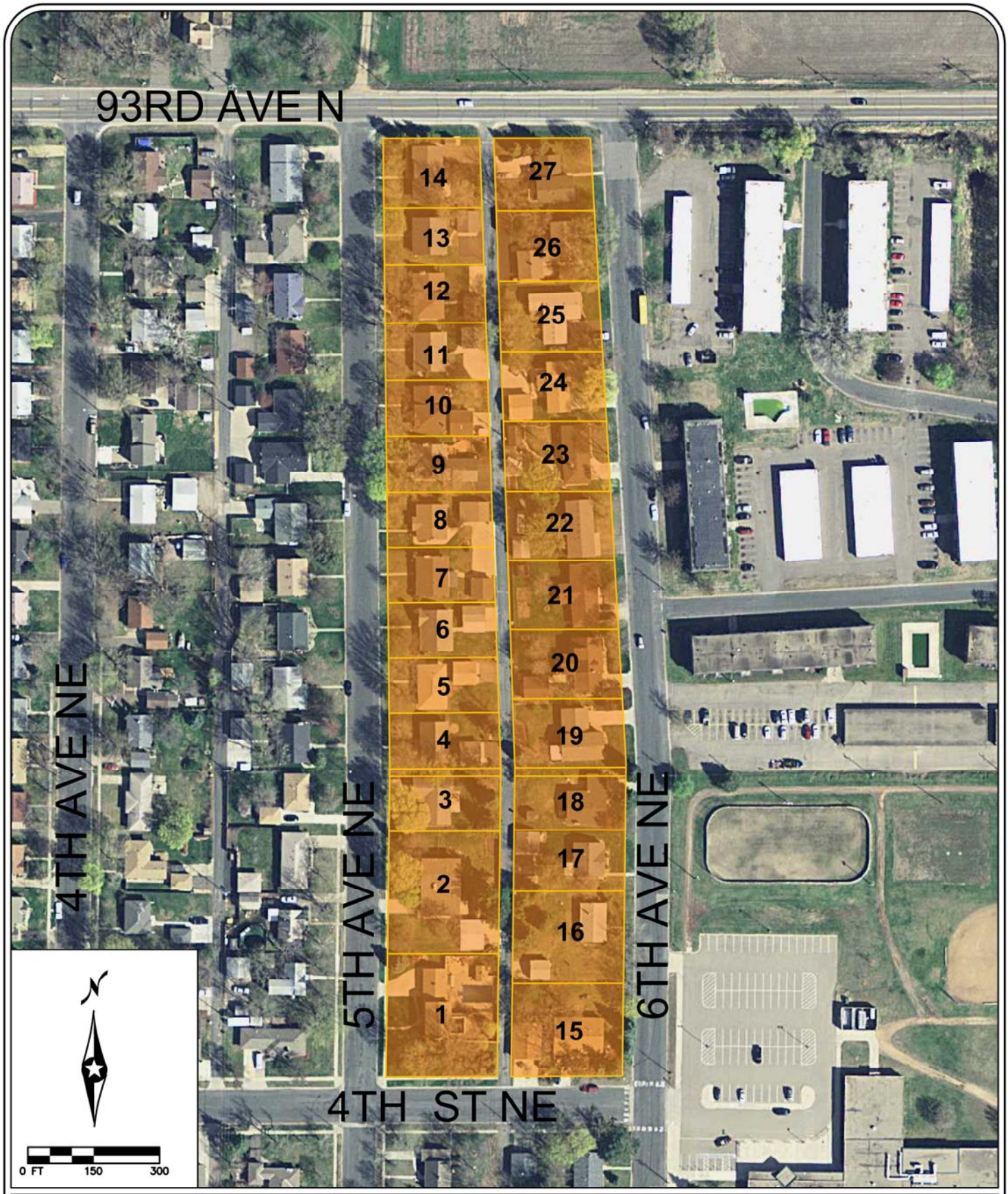
APPENDIX E

Preliminary Assessment Map
Preliminary Assessment Roll



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INFRASTRUCTURE ENGINEERING PLANNING CONSTRUCTION

2018 ALLEY RECONSTRUCTION PROJECT

OSSEO, MN

12/8/2016

PRELIMINARY ASSESSMENT MAP

CITY OF OSSEO
2018 ALLEY RECONSTRUCTION PROJECT
PRELIMINARY ASSESSMENT ROLL

Date: 11/6/2017
WSB Project No.: 03429-070

	Rate
Complete Reconstruction Residential Single-Family Per Unit Assessment:	**$6,580.00**

MAP ID	PID	FEE OWNER	FEE OWNER ADDRESS	CITY/STATE/ZIP	PROPERTY ADDRESS	USE DESCRIPTION	UNIT ASSESSMENT RATE	PROPOSED ASSESSMENT
1	1811921210043	DUANE E & AMY L POPPE	408 5TH AVE N E	OSSEO MN 55369	408 5TH AVE NE	RESIDENTIAL	1	$6,580.00
2	1811921210085	KENNETH J FELBER	424 5TH AVE N E	OSSEO MN 55369	424 5TH AVE NE	RESIDENTIAL	1	$6,580.00
3	1811921210046	SUZANNE M PACE	432 5TH AV NE	OSSEO MN 55369	432 5TH AVE NE	RESIDENTIAL	1	$6,580.00
4	1811921210035	STACY L & TIMOTHY M POLICH	500 5TH AVE N E	OSSEO MN 55369	500 5TH AVE NE	RESIDENTIAL	1	$6,580.00
5	1811921210034	KATHARINE ELIZABETH METHUM	508 5TH AVE N E	OSSEO MN 55369	508 5TH AVE NE	RESIDENTIAL	1	$6,580.00
6	1811921210033	DIANA L LAHD	516 5TH AVE N E	OSSEO MN 55369	516 5TH AVE NE	RESIDENTIAL	1	$6,580.00
7	1811921210032	LEE RITTER & SUE RITTER	524 5TH AVE N E	OSSEO MN 55369	524 5TH AVE NE	RESIDENTIAL	1	$6,580.00
8	1811921210031	ROBERT & BETTY ROBIDEAU	532 5TH AVE N E	OSSEO MN 55369	532 5TH AVE NE	RESIDENTIAL	1	$6,580.00
9	1811921210030	SALLY M HEIN	600 5TH AVE N E	OSSEO MN 55369	600 5TH AVE NE	RESIDENTIAL	1	$6,580.00
10	1811921210029	BRADLEY J ROSCH	608 5TH AVE N E	OSSEO MN 55369	608 5TH AVE NE	RESIDENTIAL	1	$6,580.00
11	1811921210028	LESLIE J EIDEN	616 5TH AVE N E	OSSEO MN 55369	616 5TH AVE NE	RESIDENTIAL	1	$6,580.00
12	1811921210027	TODD W & DAWN L TESSMAN	624 5TH AVE N E	OSSEO MN 55369	624 5TH AVE NE	RESIDENTIAL	1	$6,580.00
13	1811921210026	LISA A PETERSON	632 5TH AVE N E	OSSEO MN 55369	632 5TH AVE NE	RESIDENTIAL	1	$6,580.00
14	1811921210025	ALAN & KATHERINE KOWALCHYK	25061 BAY CEDER DRIVE	BONITA SPRINGS, FL 34134	640 5TH AVE NE	RESIDENTIAL	1	$6,580.00
15	1811921210087	CAROL EMMANS	401 6TH AVE N E	OSSEO MN 55369	401 6TH AVE NE	RESIDENTIAL	1	$6,580.00
16	1811921210086	ERIN M HENTE	417 6TH AVE N E	OSSEO MN 55369	417 6TH AVE NE	RESIDENTIAL	1	$6,580.00
17	1811921210039	MICHAEL W MACK	425 6TH AVE N E	OSSEO MN 55369	425 6TH AVE NE	RESIDENTIAL	1	$6,580.00
18	1811921210038	MOLLY BURNS	433 6TH AVE N E	OSSEO MN 55369	433 6TH AVE NE	RESIDENTIAL	1	$6,580.00
19	1811921210073	MAYNARD R EDER	501 6TH AVE N E	OSSEO MN 55369	501 6TH AVE NE	RESIDENTIAL	1	$6,580.00
20	1811921210072	TIMOTHY J PASKE	509 6TH AVE NE	OSSEO MN 55369	509 6TH AVE NE	RESIDENTIAL	1	$6,580.00
21	1811921210071	MARK R & LUJEAN K RUEL	517 6TH AVE N E	OSSEO MN 55369	517 6TH AVE NE	RESIDENTIAL	1	$6,580.00
22	1811921210070	RAYMOND/PAMELA MC DONALD JR	601 6TH AVE N	OSSEO MN 55369	601 6TH AVE NE	RESIDENTIAL	1	$6,580.00
23	1811921210069	BRUCE J DAHLHEIMER	609 6TH AV NE	OSSEO MN 55369	609 6TH AVE NE	RESIDENTIAL	1	$6,580.00
24	1811921210068	PAUL F FERGUSON	617 6TH AVE N E	OSSEO MN 55369	617 6TH AVE NE	RESIDENTIAL	1	$6,580.00
25	1811921210067	RHONDA SCHLEGEL	19705 CTY RD 30	CORCORAN MN 55340	625 6TH AVE NE	RESIDENTIAL	1	$6,580.00
26	1811921210066	KAREN L KELLAR TRUSTEE	633 6TH AVE N E	OSSEO MN 55369	633 6TH AVE NE	RESIDENTIAL	1	$6,580.00
27	1811921210065	CHRIST F & JOYCE M JENSEN	22275 CO RD NO 117	ROGERS MN 55374	641 6TH AVE NE	RESIDENTIAL	1	$6,580.00
						GRAND TOTAL - PRELIMINARY PROJECT ASSESSMENT:		***$177,660.00***

APPENDIX F

Public Comment Summary

COMMENT CARD

City of Osseo, Minnesota
2017 Alley Reconstruction Project

Date: Thursday, November 10, 2016
Location: Osseo Community Center

Name Bob + Betty Robideau. Phone (763) 424-7665
Address 532-5th AVE. NE.
City OSSEO State MN. Zip Code 55369

General Comments

Thanks for all the information on the 10th - its important that we get all correct information before work begins. Our alley Right Now is Not much more than a asphalt sidewalk - holes, bumps; many filled in pot holes. We Look forward to seeing a wonderful NEW ALLEY that is done correctly - smooth - LEVEL and done with CARE.

Signature Bob R. Robideau